# **BUILDING INFORMATION MODELLING IN STRUCTURAL**

# ANALYSIS

Finite element method



Bachelor's thesis

**Construction Engineering** 

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### ABSTRACT

The purpose of this Bachelor's thesis was to examine the applicability of Dlubal RFEM, a structural analysis software based on Finite Element method on a renovation project made in a BIM software Autodesk Revit. The aim was to make observations on the performance and collaboration of the software and produce useful information about learning to use it.

The thesis consisted of making a virtual structural model of a residential multi-storey building's frame and using it to generate a modern alternative to the 100-year-old structure with minimum changes made to the historical and architectural appearance of the building. The model was then subjected to a structural analysis where suggestions were made regarding its structural design. Throughout the project, observations were made on both Aurodesk Revit and Dlubal RFEM features and its user assistance materials.

As a result of the thesis it was concluded that Dlubal RFEM can be practical in combination with Autodesk Revit for such renovation projects, even though greatly affecting performance of computer the generated materials satisfy initial requirements and provide new opportunities for the further steps of the project.

**Keywords** BIM, CAD, FEM, FEA, renovation, Autodesk Revit, Dlubal RFEM.

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### **1** INTRODUCTION

With the rising number of buildings approaching the end of their serviceability period, renovation plays an ever-larger role in residential and commercial expansions. The goal of this Bachelor's thesis is to apply structural renovation on a multi-storey building to increase its sustainability and serviceability period, while preserving its cultural and architectural appearance. This is a stage where Building Information Modelling (BIM) plays a significant role in the construction process.

There are many BIM platforms in use today, each coming with its pros and cons. This thesis specifically explores Autodesk Revit, a powerful BIM software, and Dlubal RFEM, an easy yet powerful structural analysis program based on Finite Element method (FEM).

The thesis shows the process of structural modelling applied on an existing aged building in order to assist its potential future renovation. Additionally, it explores renovation possibilities, performance, application and collaboration of software providing insights into learning to use Autodesk Revit and Dlubal RFEM that can be useful to other people working or studying in construction industry.

This is done through observation and application of tools provided by the software. The programs are used to generate data on buildings and make suggestions on its structural design and potential renovation.

The building is of cultural and historical value located in Kallio, Helsinki. It is a five-storey residential building with an additionally constructed sixth floor in the old attic area. The goal of this project is to perform a structural renovation by replacing the old deteriorating intermediate concrete floor with a new reinforced concrete structure to prolong serviceability of the building.

### 2 BUILDING INFORMATION MODELLING

Building Information Modelling (BIM) can be depicted as a process of developing, managing and bringing into existence a digital project, which contains functional characteristics and physical properties of purposefully used facilities. Based on the results of extensive research the outcome of this process is represented in a model which nowadays is generated through a specially developed software. Apart from graphical visualisation a model includes a decisive information concerning a project cycle and every aspect of it (Cherkaoui, 2017).

Rapidly growing due to its applicability, BIM technology has become a widely discussed topic and an essential part of construction industry. Possibility to visualize and alter the existent and non-existent facilities has allowed project managers to see a construction prior its physical execution, reduce conflicting areas of the project, improve safety issues, and analyse and apply simulations of potential impacts. Additionally, it allows controlling estimation, costs, material supply and facility maintenance throughout the lifespan of a project, affecting its efficiency (Cherkaoui, 2016).

Playing a major role in BIM, Computer-Aided Design (CAD) has granted people the possibility to represent and record the shape information, which can be altered at any time, given the possession of the BIM model. That made an impact on visual communication and allowed people to learn about the progression of the erection, functionality of building components and required quantity of materials regarding an upcoming project. Instantaneous modifications to the model have preserved large amount of time spent by drafters to apply the necessary changes. It significantly raised accuracy of the drawings and production of the drafting copies (Cherkaoui, 2017).

With a wide variety of BIM platforms present today, each coming with its assets and liabilities, the choice falls on specific tools, provided by the software. Primarily used in architecture, engineering and construction (AEC), BIM software unites all three disciples in to one industry. Integration of the separate parts of one project and working together to bring it into existence has made work and collaboration more efficient.

### 3 AUTODESK REVIT

Autodesk, Inc. is a multinational software corporation founded in 1982 in the United States by John Walker. It specializes in several business fields: product design, manufacturing, architecture, engineering, construction, media, and entertainment (Funding Universe, n.d.). With the company's main office located in San Rafael, California, Autodesk is a leader in 3D design and engineering, having customers worldwide and offices established in more than thirty countries (Autodesk, 2019).

The company is best known for their CAD platforms, AutoCAD – a drafting application, and Revit – a powerful BIM software. Both are widely used by architects, engineers and structural designers to design, draft and model structures and other different objects.

Autodesk Revit was originally developed by Charles River Software, founded in 1997 by Leonid Raiz and Irwin Jungreis. It was later renamed in Revit Technology Corporation in 2000 and acquired by Autodesk in 2002.

Intending to create an architectural version of software that could handle more complex projects than ArchiCAD, Revit came out as a very powerful BIM collaboration tool, which brings the unique perspectives of different disciples into one three-dimensional model, including geometry, design and construction information (Bergin, 2012). The interface of the program can be seen in Figure 1.



Figure 1. Interface screenshot of Autodesk Revit

### 4 DLUBAL RFEM

DLUBAL software was founded as a one-man company in 1987 in Harswinkel, Germany by George Dlubal. The company is specialized in structural analysis solutions which are based on FEM. The method implies approximation of the unknown result by subdividing a large system into smaller simpler parts, running them through algebraic equations and assembling into a larger system, modelling the entire problem. Some of the interest areas of the problems are structural analysis, heat transfer, fluid flow and electromagnetic potential (Finite element method (FEM), 2019).

Ever since its establishment the company has continuously developed and optimized user-friendly and powerful programs for structural design and analysis. The most notable ones are RSTAB – an analysis and design software for beam, frame and truss structures, and RFEM – a 3D Finite Element Analysis (FEA) software for design and analysis of structures made of various materials.

Having more than 200 employees the company have established offices in 6 countries at 9 locations and is being used by more than 7800 companies

with more than 45,000 satisfied customers in 95 countries (Dlubal software, 2019).

Dlubal RFEM is a 3D FEA software operating under Microsoft Windows. The program is used to define structures, various materials such as concrete, steel, timber, glass, and various loads. RFEM consists of different corresponding add-on modules providing deformations, internal forces, stresses, support forces and soil contact stresses. Such concept allows to manually tailor a program package that suits a user's individual needs and allows to perform further analysis and design according to various standards (RFEM, 2019). The interface of RFEM is shown below in Figure 2.

The program offers numerous interfaces and a wide range of useful features for data exchange within the BIM process, maintaining all data in a digital 3D model, which is used throughout all planning stages. As a result, RFEM is a good alternative to other structural analysis software such as SAP 2000, RISA and Robot Structural analysis, which has an integration in the standard BIM software such as Revit or Tekla. Various CAD and structural analysis programs can use the same model, which can be transferred between programs.



Figure 2. Interface screenshot of DLUBAL RFEM

# 5 AS OY FRANZENINAUKIO PROJECT

### 5.1 Building information

As Oy Helsingin Franzeninaukio is a multi-storey building located on Fleminginkatu 10, Kallio, Helsinki. The original designs of the project are

dated in 1910 and the building itself was erected in 1912 by the Finnish designer and builder Heikki Kaartinen, (Figure 3). The building is of great architectural, cultural and historical value and falls under a protection class Sr-2 (suojeltava rakennus), which restricts demolition, extension and reconstruction works of any parts of the building that affect historical value or architectural features without permission from the building committee. The building has four residential floors and an attic floor. Two spacious staircases going through the building.



Figure 3. Fleminginkatu 10. Franzeninkatu 23 (Helsinki kuvia, n.d.)

During a major renovation taking place in 1980-1982, two elevator shafts were constructed on both sides of staircases, going from the ground floor to the last floor of the residential area. The elevator and ventilation engine rooms were placed on the attic. The basement is redesigned for office and storage area and cold offices in the attic were replaced with additional storage area. Improvements were made to electricity, plumbing and ventilation allowing extension of the bathrooms. During the renovation, the style of 1910s faced, roof structure, spacious stairwells and the heights of the apartments were preserved (Rakennuslupa, 2016).

A major renovation took place in 2016-2018 with major changes done to the deteriorating wooden roof structure and apartment arrangement. Previously installed electrical and HVAC systems were completely removed and replaced with a modern one. Old and deteriorating exterior plaster and ornament was removed and reapplied anew with the same architectural view as the original (Rakennuslupa, 2016).

### 5.1.1 Demolition of old structures

Renovation included construction of eight new residential apartments on the attic floor, for which to be feasible the old roof structure had to be demolished and rebuilt anew, differing as little as possible from the original architectural view. Though majority of old chimneys were preserved, slight modifications were made to them to fit the ventilation pipes. The demolition drawing of the attic can be found in Appendix 2.

The old wooden attic and residential intermediate floors were removed and emptied of the old insulation, revealing the original structure (see Figure 4). To bear the new roof structure, additional beams were placed between the structure's external and internal bearing walls and overlapping concrete slabs. The internal part of the eaves is reinforced to support the bottom of the new roof. The old staircase floor of the attic area is demolished and cast anew to match the level of the attic's new intermediate floor.



Figure 4. Demolished roof structure

5.1.2 Intermediate floor repairs

As the old wooden floor structure was removed, it was revealed that the old intermediate concrete slabs were covered with cracks, going from top to bottom of the slab at some places. Holes in some slabs also showed the



lack of reinforcement in them (see Figure 5). The slabs themselves rested on the bottom eaves of the old INP 200 beams.

Figure 5. Cracked floor slab

Since renovation required removal of the old wool, filled in the area between the slab and wooden floor, and refilling the empty space with a heavier small grain gravel, it was necessary to perform an inspection for the presence of cracks on all the concrete slabs and make necessary repairs. Old demolished structures, damaged and repaired slabs can be seen on the photos given in Appendix 1.

As the minor reparations were made to the concrete slabs, leaving uncracked ones under heavier load, it is certain that more cracking will occur, and the building will be a subject to a major structural renovation after next serviceability check (see Figure 6).



Figure 6. Repaired intermediate floor slab

# 6 BUILDING MODEL

### 6.1 Model preparation

The generation of the models in Autodesk Revit is done using the parametric modelling method, which means that the model is created based on a series of pre-programmed algorithms. Using its internal logic, the program is automatically generating elements rather than manually manipulating them (BIM Wiki, 2019). Autodesk offers numerous useful and easy to follow video lectures and tutorials to assist users in operating their programs.

The file containing original 3D BIM model is generated using architectural template in Autodesk Revit (Figure 7). It includes both architectural and structural drawings, types of structures, properties of objects and

installation guidance. The list of attached drawings can be seen in Table 1, parts of which can be found in Appendix 2.



Figure 7. As Oy Franzeninaukio 3D architectural model

Table 1. List of attached architectural and structural drawings
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02     Structural     Tvopiirustus     305     YLAPOHJA YLAPOHJA - KANTAVAT       02     Structural     Tvopiirustus     311     LEIKKAUS 1 - LAIKOPALKKI       02     Structural     Tvopiirustus     312     LEIKKAUS 2 - 2 YLEISLEIKKAUS       02     Structural     Tvopiirustus     312     LEIKKAUS 2 - 2 YLEISLEIKKAUS       02     Structural     Tvopiirustus     311     LEIKKAUS 7 - 7 HORMILAPIVIENNIT       02     Structural     Tvopiirustus     313     LEIKKAUS 7 - 7 HORMILAPIVIENNIT       02     Structural     Tvopiirustus     313     LEIKKAUS 7 - 7 HORMILAPIVIENNIT       02     Structural     Tvopiirustus     313     LEIKKAUS 3 - 3 LAPEIKKUNA HVS       02     Structural     Tvopiirustus     321     DET 1-1       02     Structural     Tvopiirustus     320     LEIKKAUS 10 - 10 JIIRI       02     Structural     Tvopiirustus     322     DET 2-2       02     Structural     Tvopiirustus     324     DET 4-4       02     Structural     Tvopiirustus     325     DET 5-5       02     Structural     Tvopiirustus     326     DET 7-7       02     Structural     Tvopiirustus     327     DET 7-7       02     Structural     Tvopiirustus <td>02 Structural</td> <td>Tvöniirustus</td> <td>304</td> <td>ULLAKON VALIPOHJA</td>	02 Structural	Tvöniirustus	304	ULLAKON VALIPOHJA
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O2         Structural         Tvobiliustus         S15         LEikKAUS 5 - 5 TORNI           02         Structural         Tvobiliustus         S17         LEikKAUS 7 - 7 HORMIAPIVIENNIT           02         Structural         Tvobiliustus         S14         LEikKAUS 7 - 7 HORMIAPIVIENNIT           02         Structural         Tvobiliustus         S14         LEikKAUS 4 - 4 LAPEIKKUNA HVS           02         Structural         Tvobiliustus         S13         LEikKAUS 3 - 3 LAPEIKKUNA           02         Structural         Tvobiliustus         S19         LEikKAUS 9 - 9 MÄRKÄTILA           02         Structural         Tvobiliustus         S10         LEikKAUS 9 - 9 MÄRKÄTILA           02         Structural         Tvobiliustus         S16         LEikKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvobiliustus         S23         DET 3-3           02         Structural         Tvobiliustus         S24         DET 4-4           02         Structural         Tvobiliustus         S26         DET 6-6           02         Structural         Tvobiliustus         S27         DET 1-7.7           02         Structural         Tvobiliustus         S28         DET 19-9           02<	02 Structural	Tvôniirustus	312	LEIKKAUS 2 - 2 YEEISEEIKKAUS
O2         Structural         Tvooiirustus         317         LEikKAUS 7. 7 HORMILAPIVIENNIT           02         Structural         Tvooiirustus         314         LEIKKAUS 7. 7 HORMILAPIVIENNIT           02         Structural         Tvooiirustus         313         LEIKKAUS 7. 7 HORMILAPIVIENNIT           02         Structural         Tvooiirustus         313         LEIKKAUS 3. 4. 4 LAPEIKKUNA           02         Structural         Tvooiirustus         321         DET. 1-1           02         Structural         Tvooiirustus         321         DET. 1-1           02         Structural         Tvooiirustus         321         DET. 1-1           02         Structural         Tvooiirustus         320         LEIKKAUS 9. 9 MÄRKÄTILA           02         Structural         Tvooiirustus         322         DET. 2-2           02         Structural         Tvooiirustus         324         DET. 4-4           02         Structural         Tvooiirustus         326         DET. 5-5           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvo	02 Structural	Tvôniirustus	315	LEIKKAUS 5 - 5 TORNI
02       Structural       Tvooiirustus       314       LEikKAUS 4 - 4 LAPEIKKUNA HVS         02       Structural       Tvooiirustus       313       LEIKKAUS 3 - 3 LAPEIKKUNA         02       Structural       Tvooiirustus       321       DET 1-1         02       Structural       Tvooiirustus       321       DET 1-1         02       Structural       Tvooiirustus       319       LEIKKAUS 9 - 9 MARKATILA         02       Structural       Tvooiirustus       320       LEIKKAUS 10 - 10 JIIRI         02       Structural       Tvooiirustus       322       DET 2-2         02       Structural       Tvooiirustus       323       DET 3-3         02       Structural       Tvooiirustus       324       DET 4-4         02       Structural       Tvooiirustus       325       DET 5-5         02       Structural       Tvooiirustus       327       DET 7-7         02       Structural       Tvooiirustus       329       DET 9-9         02       Structural       Tvooiirustus       332       DET 19-1         02       Structural       Tvooiirustus       333       ULLAKON PURKUPIRUSTUS         02       Structural       Tvooiirustus	02 Structural	Tvôniirustus	317	LEIKKAUS 7 - 7 HORMILÄPIVIENNIT
O2         Structural         Tvooiirustus         313         LEIKKAUS 3 - 3 LAPEIKKUNA           02         Structural         Tvooiirustus         321         DET 1-1           02         Structural         Tvooiirustus         321         DET 1-1           02         Structural         Tvooiirustus         319         LEIKKAUS 9 - 9 MÄRKÄTILA           02         Structural         Tvooiirustus         320         LEIKKAUS 9 - 6 MÄRKÄTILA           02         Structural         Tvooiirustus         320         LEIKKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvooiirustus         322         DET 2-2           02         Structural         Tvooiirustus         324         DET 4-4           02         Structural         Tvooiirustus         326         DET 5-5           02         Structural         Tvooiirustus         327         DET 7-7           02         Structural         Tvooiirustus         329         DET 9-9           02         Structural         Tvooiirustus         321         DET 12-12           02         Structural         Tvooiirustus         332         DET 12-12           02         Structural         Tvooiirustus         333 </td <td>02 Structural</td> <td>Tvöniirustus</td> <td>314</td> <td>LEIKKAUS 4 - 4 LAPEIKKUNA HVS</td>	02 Structural	Tvöniirustus	314	LEIKKAUS 4 - 4 LAPEIKKUNA HVS
O2         Structural         Tvopiirustus         321         DET. 1-1           02         Structural         Tvopiirustus         319         LEIKKAUS 9 - 9 MÄRKÄTILA           02         Structural         Tvopiirustus         310         LEIKKAUS 10 - 10 JIIRI           02         Structural         Tvopiirustus         320         LEIKKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvopiirustus         322         DET. 2-2           02         Structural         Tvopiirustus         323         DET. 3-3           02         Structural         Tvopiirustus         324         DET. 4-4           02         Structural         Tvopiirustus         325         DET. 5-5           02         Structural         Tvopiirustus         326         DET. 7-7           02         Structural         Tvopiirustus         327         DET. 7-7           02         Structural         Tvopiirustus         329         DET. 9-9           02         Structural         Tvopiirustus         332         DET. 11-11           02         Structural         Tvopiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvopiirustus         333	02 Structural	Tvöniirustus	313	LEIKKAUS 3, 3 LAPEIKKUNA
02         Structural         Tvooiirustus         319         LEIKKAUS 9 - 9 MÄRKÄTILA           02         Structural         Tvooiirustus         320         LEIKKAUS 10 - 10 JIRI           02         Structural         Tvooiirustus         316         LEIKKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvooiirustus         322         DET. 2-2           02         Structural         Tvooiirustus         323         DET. 3-3           02         Structural         Tvooiirustus         324         DET. 4-4           02         Structural         Tvooiirustus         326         DET. 6-6           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         328         DET. 8-8           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         ULAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318	02 Structural	Tvöniirustus	321	DFT 1-1
02         Structural         Tvopiirustus         320         LEikKAUS 10 - 10 JiiRi           02         Structural         Tvopiirustus         316         LEikKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvopiirustus         322         DET 2-2           02         Structural         Tvopiirustus         323         DET 3-3           02         Structural         Tvopiirustus         324         DET 4-4           02         Structural         Tvopiirustus         324         DET 4-4           02         Structural         Tvopiirustus         326         DET 5-5           02         Structural         Tvopiirustus         327         DET 7-7           02         Structural         Tvopiirustus         329         DET 9-9           02         Structural         Tvopiirustus         332         DET 12-12           02         Structural         Tvopiirustus         333         DET 12-12           02         Structural         Tvopiirustus         333         DET 12-12           02         Structural         Tvopiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvopiirustus         318         LEIK	02 Structural	Tvöpiirustus	319	LEIKKAUS 9 - 9 MÄRKÄTILA
02         Structural         Tvooiirustus         316         LEIKKAUS 6 - 6 KATTOIKKUNA PERIA           02         Structural         Tvooiirustus         322         DET. 2-2           02         Structural         Tvooiirustus         323         DET. 3-3           02         Structural         Tvooiirustus         324         DET. 4-4           02         Structural         Tvooiirustus         325         DET. 5-5           02         Structural         Tvooiirustus         326         DET. 6-6           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         328         DET. 7-7           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         331         DET. 12-12           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318         LEIKKAUS 8-8         PORRASKAYTÄVÄN           02         Structural         Tvooiirustus	02 Structural	Tvöpiirustus	320	LEIKKAUS 10 - 10 JIIRI
02         Structural         Tvodiirustus         322         DET         2-2           02         Structural         Tvodiirustus         323         DET         3-3           02         Structural         Tvodiirustus         324         DET         4-4           02         Structural         Tvodiirustus         326         DET         6-6           02         Structural         Tvodiirustus         326         DET         6-6           02         Structural         Tvodiirustus         327         DET         7-7           02         Structural         Tvodiirustus         329         DET         9-9           02         Structural         Tvodiirustus         321         DET         1-9           02         Structural         Tvodiirustus         332         DET         1-9           02         Structural         Tvodiirustus         332         DET         12-12           02         Structural         Tvodiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvodiirustus         318         LEIKKAUS & 8 PORRASKAYTÄVÄN           02         Structural         Tvodiirustus         309         U	02 Structural	Tvöniirustus	316	LEIKKAUS 6 - 6 KATTOIKKUNA PERIA
02         Structural         Tvobiirustus         323         DET 3-3           02         Structural         Tvobiirustus         324         DET 4-4           02         Structural         Tvobiirustus         325         DET 5-5           02         Structural         Tvobiirustus         325         DET 5-5           02         Structural         Tvobiirustus         326         DET 7-7           02         Structural         Tvobiirustus         327         DET 7-7           02         Structural         Tvobiirustus         328         DET 8-8           02         Structural         Tvobiirustus         329         DET 19-9           02         Structural         Tvobiirustus         332         DET 12-12           02         Structural         Tvobiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvobiirustus         333         ULLAKON PALOKATKAN           02         Structural         Tvobiirustus         309         ULLAKKO PALOKATKAN           02         Structural         Tvobiirustus         335         PALOKATKODETAL JIT	02 Structural	Tvöniirustus	322	DFT 2-2
02         Structural         Tvooiirustus         324         DET. 4-4           02         Structural         Tvooiirustus         325         DET. 5-5           02         Structural         Tvooiirustus         326         DET. 6-6           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         328         DET. 7-7           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318         LEIKKAUS 8-8 PORRASKÄYTÄVÄN           02         Structural         Tvooiirustus         309         ULLAKKO. PALOKATKOT           02         Structural         Tvooiirustus         335         PALOKATKODETAI JIT	02 Structural	Tvöpiirustus	323	DET 3-3
02         Structural         Tvooiirustus         325         DET. 5-5           02         Structural         Tvooiirustus         326         DET. 6-6           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         328         DET. 8-8           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         331         DET. 12-12           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         UL LAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         333         UL LAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         309         UL LAKON PALOKATKAVAN           02         Structural         Tvooiirustus         309         UL LAKON PALOKATKODETAL JIT           02         Structural         Tvooiirustus         335         PALOKATKODETAL JIT	02 Structural	Tvöniirustus	324	DET 4.4
02         Structural         Tvooiirustus         326         DET. 6-6           02         Structural         Tvooiirustus         327         DET. 7-7           02         Structural         Tvooiirustus         328         DET. 8-8           02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         331         DET. 11-11           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318         LEIKKAUS 8-8 PORRASKÄYTÄVÄN           02         Structural         Tvooiirustus         309         ULLAKKO. PALOKATKOT           02         Structural         Tvooiirustus         335         PALOKATKODETAI JIIT	02 Structural	Tvöniirustus	325	DET 5-5
02         Structural         Tvopiirustus         327         DET. 7.7           02         Structural         Tvopiirustus         328         DET. 8-8           02         Structural         Tvopiirustus         329         DET. 9-9           02         Structural         Tvopiirustus         331         DET. 12-9           02         Structural         Tvopiirustus         332         DET. 12-12           02         Structural         Tvopiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvopiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvopiirustus         318         LEIKKAUS 8-8 PORRASKAYTÄVÄN           02         Structural         Tvopiirustus         309         ULLAKKO. PALOKATKOT           02         Structural         Tvopiirustus         330         DET. 10-10           02         Structural         Tvopiirustus         335         PALOKATKODETALJIT	02 Structural	Tvöpiirustus	326	DET 6-6
02     Structural     Tvooiirustus     328     DET. 8-8       02     Structural     Tvooiirustus     329     DET. 9-9       02     Structural     Tvooiirustus     331     DET. 11-11       02     Structural     Tvooiirustus     332     DET. 12-12       02     Structural     Tvooiirustus     333     ULLAKON PURKUPIIRUSTUS       02     Structural     Tvooiirustus     318     LEIKKAUS 8 - 8 PORRASKAYTÄVÄN       02     Structural     Tvooiirustus     309     ULLAKKO. PALOKATKOT       02     Structural     Tvooiirustus     335     DET. 10-10       02     Structural     Tvooiirustus     335     DET. 10-11	02 Structural	Työpiirustus	327	DFT 7-7
02         Structural         Tvooiirustus         329         DET. 9-9           02         Structural         Tvooiirustus         331         DET. 11-11           02         Structural         Tvooiirustus         332         DET. 12-12           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318         LEIKKAUS 8-8 PORRASKÄYTÄVÄN           02         Structural         Tvooiirustus         309         ULLAKKO. PALOKATKOT           02         Structural         Tvooiirustus         335         DET. 10-10           02         Structural         Tvooiirustus         335         PALOKATKODETALJIT	02 Structural	Työpiirustus	328	DFT 8-8
02 Structural     Tvooiirustus     331     DET. 11-11       02 Structural     Tvooiirustus     332     DET. 12-12       02 Structural     Tvooiirustus     333     ULLAKON PURKUPIIRUSTUS       02 Structural     Tvooiirustus     318     LEIKKAUS 8 - 8 PORRASKAYTÄVÄN       02 Structural     Tvooiirustus     318     ULLAKKO. PALOKATKOT       02 Structural     Tvooiirustus     309     ULLAKKO. PALOKATKOT       02 Structural     Tvooiirustus     330     DET. 10-10       02 Structural     Tvooiirustus     335     PALOKATKODETALJIT	02 Structural	Työpiirustus	329	DFT 9-9
O2         Structural         Tvooiirustus         332         DET: 12-12           02         Structural         Tvooiirustus         333         ULLAKON PURKUPIIRUSTUS           02         Structural         Tvooiirustus         318         LEIKKAUS 8 - 8 PORRASKÄYTÄVÄN           02         Structural         Tvooiirustus         309         ULLAKKO. PALOKATKOT           02         Structural         Tvooiirustus         309         DET: 10-10           02         Structural         Tvooiirustus         335         PALOKATKODETAL JIT	02 Structural	Työpiirustus	331	DET 11-11
02 Structural Tvooiirustus 333 ULLAKON PURKUPIIRUSTUS 02 Structural Tvooiirustus 318 LEIKKAUS 8 - 8 PORASKAYTÄVÄN 02 Structural Tvooiirustus 309 ULLAKKO PALOKATKOT 02 Structural Tvooiirustus 330 DET 10-10 02 Structural Tvooiirustus 335 PALOKATKODETALJIT	02 Structural	Työpiirustus	332	DET 12-12
02 Structural Tvopiirustus 318 LEIKKAUS 8 - 8 PORRASKAYTÄVÄN 02 Structural Tvopiirustus 309 ULLAKKO. PALOKATKOT 02 Structural Tvopiirustus 330 DET. 10-10 02 Structural Tvopiirustus 335 PALOKATKODETAI JIT	02 Structural	Työpiirustus	333	ULLAKON PURKUPIRUSTUS
02 Structural Tvooiirustus 309 ULLAKKO PALOKATKOT 02 Structural Tvooiirustus 330 DET. 10-10 02 Structural Tvooiirustus 335 PALOKATKODETALJIT	02 Structural	Työpiirustus	318	I FIKKAUS 8 - 8 PORRASKAYTAVAN
02 Structural Tvöpiirustus 330 DET. 10-10 02 Structural Tvöpiirustus 335 PALOKATKODETAI JIT	02 Structural	Työpiirustus	309	<b>ULLAKKO PALOKATKOT</b>
02 Structural Tyopiirustus 335 PALOKATKODFTALJIT	02 Structural	Työpiirustus	330	DFT 10-10
	02 Structural	Työpiirustus	335	PAI OKATKODETAL JIT

Following the given structural drawings, the building is retraced on the structural template, as seen in Figure 8. All objects related to architectural representation are removed, changing architecturally generated structural objects to analytical structural objects. This is done to obtain a simplified analytical model of the building which is used for structural analysis. Revit provides various properties applicable to any generated objects as well as a great choice of structural elements, which can be withdrawn from the library or generated manually.



Figure 8. 3D structure of the old building

### 6.2 Structural modelling

Due to the failure of the old concrete floors and stress on the bricks from the edges of the INP200 beams, the old intermediate floor structure is removed in this project and a more durable supporting frame added to the building, to increase its sustainability (see Figure 9). The frame consists of 380x380mm reinforced concrete columns, 300x300mm reinforced concrete beams and 150mm reinforced concrete floor.

To aid reinforcing process Revit provides a variety of stiffening shapes for presentation. Stiffening is applied approximately during the design stage of the project; it is afterwards analysed with the whole building and corrections are made in case of failure of the element.



Figure 9. Generated supporting frame

### 6.3 Exporting the model

The prepared analytical model is displayed in an easy and comprehensive building structure which includes physical and analytical properties of elements in detail, as seen in Figure 10 on the next page. Prior structural analysis, the model is exported to RFEM. If both programs are installed, Revit makes the access of RFEM's interface directly available through its interface.

The elements and their properties can be manually selected or limited to the model and/or load data in exporting properties (see Figure 11). This is useful when defined elements are set in an external program, but only specific properties are needed be imported into the original program.



## Figure 10. Analytical model of the building



Figure 11. Revit Structure – Dlubal Link

# 7 STRUCTURAL ANALYSIS

7.1 Conflicts after transfer

Unlike Revit, RFEM uses Object-oriented programming (OOP) a concept of objects, which contain data and code in the form of procedures (Technopedia, 2012). During the export the existing RFEM data is checked and information is added. An equivalent object is added for beam, column, floor or wall, not just collection of lines and surfaces. Due to adjustable reference lines, the load bearing elements are merged in an optimal calculation model, connecting centre points of intersecting beams and columns. The exported model is presented in Figure 12.



Figure 12. RFEM analytical model

Prior to running the structural analysis on the building, the model is subjected to issue checks. The model check allows to search and reveal any occurring problems or conflicts, be that identical nodes, overlapping lines, members or surfaces, or crossing unconnected lines and members. The problems are shown on the model, as seen in Figure 13 and then are corrected manually. The plausibility check of entries is done to see if calculation would be feasible.

Structures consisting of steel, reinforced concrete and timber can be designed using tools provided by RFEM. The list of materials is found in Materials table and is marked with a comment if the material was imported or used from a programs database. Material table can be seen in Figure 14.



Figure 13. Crossing unconnected members

	A	В	С	D	E	F	G	Ĥ	Í
Material	Material	Modulus of Elasticity	Shear Modulus	Poisson's Ratio	Specific Weight	Coeff. of Th. Exp.	Partial Factor	Material	
No.	Description	E [kN/cm <sup>2</sup> ]	G [kN/cm <sup>2</sup> ]	v [-]	γ [kN/m <sup>3</sup> ]	α.[1/°C]	γM [-]	Model	Comment
2	Concrete C25/30   EN 1992-1-1:2004/A	3100.00	1291.67	0.200	25.00	1.00E-05	1.00	Isotropic Linear Elastic	Revit material constant:
3	Concrete C25/30   EN 1992-1-1:2004/A	3100.00	1291.67	0.200	25.00	1.00E-05	1.00	Isotropic Linear Elastic	Revit material constants
4	Concrete C30/37   EN 1992-1-1:2004/A	3300.00	1375.00	0.200	25.00	1.00E-05	1.00	Isotropic Linear Elastic	
5	Masonry	2325.00	996.14	0.167	24.07	1.00E-05	1.00	Isotropic Linear Elastic	Revit material constants
6									
7									
8									
9									
10									
11									
12									
13									
14									

Figure 14. Material table of exported building

7.2 International technical standards

Dlubal RFEM implements numerous international technical standards describing different design situations. These situations determine the expected conditions of the building during its construction and usage. The following standards amongst others include Eurocodes, American Standards, Russian Standards, Chinese Standards, Indian Standards and Brazilian Standards. Load and result combinations generated on the analytical model during this project are done according to SFS-EN 1990 Eurocode – Basis of structural design. Information about design situation is shown in Figure 15 and Figure 16.

Existing C	ombination Expressions	CE No.	Combination Expression Desc	ription		Use					
STR CE1	ULS	1	ULS		×						
SCh CE2	SLS	General Ontio	ins for Combinations								
SHI CES	SLS			-	001050						
aug (E4	SLS	Design Situati	Design Stituation EN 1990 S								
		BIBULS (STR	~ 0								
		ECU ULS (EQU	ULS (EQU) - Permanent / transient								
		ACC ULS (EQL	J) - Accidental								
		BIB ULS (EQU	J) - Selsmic								
		STRIULS (STR	STR ULS (STR/GEO) - Permanent / transient - Eq. 6.10a and 6.10b								
		ERECULS (STR	RIR ULS (STR/GEO) - Accidental								
		KIN SIS - Cha	SCH SLS - Characteristic								
		SET SLS - Free	nuent								
		SCO SI S - Qua	Star SLS - Quasi-permanent								
		Reducing n Examining Selecting le	umber of load cases results aading variable actions	Result Combinations Generate additional (result envelopes) Generate additional each combination e	y Either/Or y a separat pression	r result combination te Either/Or result combin	nation f				
		Numbering of	Generated Combinations	Generated Load Comb	inations						
		First ourses	of generated	Method of analysis:	Second	-order analysis (P-Delta)	1				
		Load combinat	tions: 2	Generated Action Com	binations						
		Result combin	ations: 1 🜩	List and number:	AC1	AC28 (28/62)	0				
1	1	Comment									

Figure 15. Design situations

EN 1990 | SFS ULS (STR/GEO) - Permanent / transient - Eq. 6.10a and 6.10b  $\sum_{j\geq 1} \gamma_{G,j}\, G_{k,j} + \, \gamma_P P$  $\sum_{j \geq 1} \xi_{j} \gamma_{G,j} G_{k,j} + \gamma_{P} P + \gamma_{Q,1} Q_{k,1} + \sum_{i > 1} \gamma_{Q,i} \psi_{0,i} Q_{k,i}$ γG,j : Partial factor for permanent actions Gk,j : Permanent actions γP : Partial factor for prestress actions P> : Prestressing action  $\gamma_{Q,1}$  : Partial factor for leading variable action Qk,1 : Leading variable action : Partial factor for non-leading variable actions γQ,i  $\psi_{0,i}$  : Coefficient for combination value Qk,i : Other variable actions : Reduction coefficient ξį

Figure 16. Information about design situation

7.3 Loads and Load combinations

Dlubal RFEM provides essential tools for generating wind, snow, surface, and member loads and automatically calculates the self-weights of all elements and total mass of the building (see Figure17). Generated load cases are manually assigned to an action category, which includes partial safety factors and combination coefficients (Figure 18). The loads are then applied to desired elements and are visible in a Load case table as seen in Figure 19 on the next page.

isting Mass Cases	MC No.	Mass Case Description				
MC1 MC1	1	MC1		12	-	
	General					
	Mass Cat	se Туре		Sum of Masses		
	Perm	anent	~	Self-weight:	7201726.08	[kg]
	Masses			Components of LC/CO		[kg]
	From	self-weight of structure		Additional masses at		
	From	force components of:		Nodes:		[kg]
	() Lo	ad case:		Línes:		
	<b>a</b> (	C1 - FloorDL	~	Members:		[kg]
	OLO	ad combination:		Surfaces:		[kg]
	C	01 - Load Combination 1	~	Total mass:	7201726.08	[kg]
	Manually	define additional masses at:			L	
	Nodes			Center of total mass	2	ſm
	☐ Unes ☐ Memb ☐ Surfac	ers es			.*	
						Calculate Masse
	Comment					
				v (7	2	

Figure 17. Sum of masses

xisting Act	on Combinations		AC No.		Action Go	ombination Descri	ption					Use	
AC1	1.35G	^		5	1.15G +	1.50QiA + 0.90Q	w + 1.05Qs				0		
IN AC2	0.90G												
EDA IN	1.15G		Genera	ξη/									
AC4	1.15G + 1.50QIA		Design	Situation						EN	1990   SFS		
AC5	1.15G + 1.50QIA + 0.90QW		ISTER UT	S (STR/GE	O) - Permane	ent / transient - Eq	. 6.10a and 6	5.10b			A		
AC6	1.15G + 1.50QIA + 0.90Qw + 1.05Qs												
AC7	1.15G + 1.50QIA + 1.05Qs		Actions	s in Action	Combination	AC6						1	
ACS	1.15G + 1.50Qw		NO.	Factor	Action	Description	Leading	ζ, D. DE	7	Ų	1.01	Load Cases	
AC9	1.15G + 1.05QIA + 1.50Qw		2	1.500	074 A2	Imposed		0.05	1.50		LC2		
AC10	1.15G + 1.05QIA + 1.50Qw + 1.05Qs		3	0.900	A4	Wind	Ä		1.50	0.60	LC4 LC7		
AC11	1.15G + 1.50Qw + 1.05Qs		4	1.050	IN AS	Snow	Ō		1.50	0.70	LC8		
AC12	1.15G + 1.50Qs												
AC13	1.15G + 1.05QIA + 1.50Qs												
AC14	1.15G + 1.05QIA + 0.90Qw + 1.50Qs												
AC15	1.15G + 0.90Qw + 1.50Qs												
AC16	0.90G												
AC17	0.90G + 1.50QIA												
AC18	0.90G + 1.50QIA + 0.90Qw												
AC19	0.90G + 1.50QlA + 0.90Qw + 1.05Qs												
AC20	0.90G + 1.50QIA + 1.05Qs												
AC21	0.90G + 1.50Qw												
AC22	0.90G + 1.05QIA + 1.50Qw												
AC23	0.90G + 1.05QIA + 1.50Qw + 1.05Qs												
AC24	0.90G + 1.50Qw + 1.05Qs												
AC25	0.90G + 1.50Qs												
AC26	0.90G + 1.05QIA + 1.50Qs												
AC27	0.90G + 1.05QIA + 0.90Qw + 1.50Qs												
AC28	0.90G + 0.90Qw + 1.50Qs						1. J. J.						
AC29	1.00G		Genera	ted	List and	f number		_					
AC30	1.00G + 1.00QiA		load co	mbinations	; CO8 .	CO11 (4/136)		>					2
AC31	1.00G + 1.00QIA + 0.60QW	~	Comm										
		>	Comm	unc									

Figure 18. Action combinations

Existing Lo	oad Cases	LC No.	Load Case Description		To Solve		
LC1	FloorDL	8	8 Snow ~				
LC2	FloorLL	The second secon					
LC4	WindLx+	General Calcu	lation Parameters				
LC5	WindLy+	Action Catego	ory	EN 1990   SFS			
LC6	WindLx-	Snow - s-	k < 2.75 kN/m^2	v			
LC7	WindLy-	Colf Minishi					
CSULC8	Snow	Sell-vveight					
		Active					
		Factor in d	irection:				
		X: (					
		Y: (	J.000 - [-]				
		z :	0.000 ∓ [-]				
		-					
		Comment					
1		)					

Figure 19. Load cases

After entering type and magnitude of the load, it is assigned to nodes, lines, members or surfaces as seen in Figure 20. The values of permanent loads and properties of elements are set with estimation to allow a margin of error, which will allow future alteration of the model to exceed its parameters. Acting loads on the structure can be seen in Appendix 3.



Figure 20. First floor Dead Load

Generation of snow and wind loads in RFEM is done with no additional modules. When the required parameters are filled, the program generates specific load automatically. Required parameters include wind and snow load zones, topography, terrain and roof geometry (Figure 21). Additional snow loads such as snow drifts are considered, too (see Figure 22).



Figure 21. Wind loads



Figure 22. Snow drift loads

Taking the partial safety factor into account, the program combines assigned load cases into one big load case and subsequently calculates it.

7.4 Finite element analysis

Analysing a phenomenon with FEM is referred to as FEA which breaks down the structure into many small substructures, finite elements. The generated structure which is identical to the original structure is referred to as the Finite Element (FE) mesh Figure 23.



Figure 23. Finite Element mesh

The size or the number of finite elements affects the precision of results. The smaller the elements are set; the closer the analysis gets to the ideal result. The proportional relation also affects the duration of the analysis.

# 8 STRUCTURAL ANALYSIS RESULTS

### 8.1 Result summary

While performing FE analysis it is important to remember that it is an approximation. The results should always be checked and clarified with engineering expertise. When the analysis is complete the results are highlighted in different colours, scaling values of internal forces and deformations from minimum to extreme, allowing visual result evaluation (see Figure 24).



Figure 24. Visual result evaluation

RFEM provides a summary of the results sorted by load cases and combinations as shown in Table 2 on the next page. The overview shows maximum displacements and rotations related to the global axes, as well as the total largest displacement. The deviation in each direction should be less than 1%, signifying that no numerical problems related to stiffness difference or structure stability have occurred. The summary is completed by selected calculation parameters of the analysis core and specifications of calculations. Different views of analysed structure can be seen in Appendix 4.

Description	B	Linit	Commont
ELO1 ELO1	vaiue	Unit	Comment
Current Control Current Control Current Control Current Control Current Curren	0.00	LAT	10
Sum of roads in X	0.00	KIN	
Sum of support forces in X	0.00	KIN	
Sum of roads in 1	0.00	KIN	
Sum of support forces in f	0.00	KIN	
Sum of loads in Z	-93990.00	KIN	Deviation: 0.00 %
Sum of support forces in 2	-93990.00	KIN	At contex of gravity of model (V: 6 690 X: 4 005 7: 10 395 m)
Resultant of reactions about X	10901 400	KINIII	At center of gravity of model
Resultant of reactions about 7	0.010	KINITI	At center of gravity of model
Maximum displacement in X direction	-0.010	KINITI	Member No. 590, yr 2,070 m
Maximum displacement in X-direction	-0.4		Member No. 309, x: 2,070 m
Maximum displacement in 7 direction	0.0	mm	Member No. 703, X 2.413 III Member No. 60, x 2.000 m
Maximum displacement in Z-direction	-70.7	mm	Member No. 60, x: 2.090 m
Maximum retation about X axis	10./	mrad	Member No. 60, x 2.090 m
Maximum rotation about X axis	13.4	mrad	Member No. 00, X. 2.090 III Member No. 955, y: 0.000 m
Maximum rotation about 7 axis	-0.0	mrad	Member No. 355, X. 0.000 m
Method of applying	U.J	mau	Geometrically Linear Analysis
Deduction of altifferen	Linear		Geometrically Linear Analysis
Number of load ingroments	1		
Number of load increments	1		
Maximum value of element of stiffness matrix on dia	2 108E±12		
Minimum value of element of stiffness matrix on diag	4 748E±04		
Infinity Norm	4.740E+04		2
Interementally increasing leading	J.235L+12		-
ELO2 Electure			
Cum of loads in V	0.00	LAL	
Sum of support forces in X	0.00	KIN LAN	
Sum of leads in X	0.00	KN L	
Sum of support forces in X	0.00	IN IN	
Sum of leads in 7	6443.30	KIN IKN	
Sum of support forces in 7	6443.29	KIN LAN	Doviation: 0.00%
Deputtent of reactions shout Y	2959 790	KIN kAlm	At conter of gravity of model (X: 6.690, X: 4.005, 7: 10.295 m)
Resultant of reactions about X	2533.700	kNim	At center of gravity of model
Posultant of reactions about 7	0.000	kNm	At center of gravity of model
Maximum displacement in X direction	0.000	NINIII mm	Member No. 589, v: 2.070 m
Maximum displacement in X-direction	0.0	mm	Member No. 703, x: 2.070 m
Maximum displacement in 7 direction	_0.1	mm	EE Moch Node No. 465967 (Y: 4 386, Y: -10 570, 7: 20 440 m)
Maximum displacement in Z-direction	-0.7	mm	FE Mech Node No. 465967 (X: 4.366, 1: -18.570, 2: 20.440 m)
Maximum vectorial displacement	0.7	mrod	E Moch Nodo No. 627221 (V. 4.969, V. 16 105, 7, 7.100 m)
Maximum rotation about X axia	0.2	mrad	E Moch Nodo No. 627275 (V: 2.691 V: 10.570, 7: 7.100 m)
Maximum rotation about 7 axis	0.3	mrad	Member No. 224, v: 0.000 m
Maximum rotation about Z-axis	U.U	maa	Cometricelly Linear Analysis
Peduation of atiffness	Linear		Geometrically Linedi Andiysis
Number of load increments		-	
Number of Ioad Increments		-	
Number of iterations	2 100E, 12		
waximum value of element of stiffness matrix on dia	2.198E+12		
Minimum value or element or sumness matrix on diag	4./40E+04		/
	J.29JL+12		

#### Table 2. Results – Summary

#### 8.2 Deformations

The display of surface and member displacements and rotations are presented both graphically and numerically. Deformations of elements can be determined for members, surfaces and solids and are related either to the local coordinate system of selected element or to global coordinate system. Accessed through Result navigator, deformations have the following meanings: U for displacement and  $\phi$  for rotation (see Table 3). Locally selected deformations list deformations of each member separately and follows as start and end of the member, division points and extreme values, where surface deformations are defined by FE meshes grid points which represent its characteristics, as seen in Figure 25.

## Table 3. Global deformations

	A	P	C	D	E	F	C		1	1	-
iber	Node	Location	U.	U	)isplacements (m	r i	<u> </u>	Rotations (mrad)		J	
	No.	x [m]		ux	uY	uz	φX	ωΥ	ωZ	Corresponding Load Cases	
	2540	0.000	max	0.0	0.0	-6.3	1.3	2.0	0.0	5	
			min	0.0	0.0	-6.3	1.3	2.0	0.0		
	115	2.090	max	0.0	0.0	-9.3	1.5	2.0	0.0		
			min	0.0	0.0	-9.3	1.5	2.0	0.0		
	2540	0.000	Max uX	0.0	0.0	-6.3	1.3	2.0	0.0	CO 98	
	115	2.090	Min ux	0.0	0.0	-9.3	1.5	2.0	0.0	CO 89	
	2540	0.000	Max uY	0.0	0.0	-6.3	1.3	2.0	0.0	CO 89	
_	2540	0.000	Min uY	0.0	0.0	-6.3	1.3	2.0	0.0	CO 98	
_	2540	0.000	Maxuz	0.0	0.0	-6.3	1.3	2.0	0.0	CO 87	
_	115	2.090	Min uZ	0.0	0.0	-9.3	1.5	2.0	0.0	CO 85	
_	115	2.090	Max φX	0.0	0.0	-9.3	1.5	2.0	0.0	CO 84	
_	2540	0.000	Μαχ φχ	0.0	0.0	-0.3	1.3	2.0	0.0	CO 90	
-	2540	0.000	Min ov	0.0	0.0	-0.3	1.3	2.0	0.0	CO 90	
	2540	0.000	Max oz	0.0	0.0	-6.3	1.3	2.0	0.0	CO 90	
	2540	0.000	Min oZ	0.0	0.0	-63	13	20	0.0	CO 97	
	2529	0.000	max	0.0	0.0	-48.8	13.2	-4.9	0.0		
			min	0.0	0.0	-48.8	13.2	-4.9	0.0		
	114	2.090	max	0.0	0.0	-76.7	13.4	-4.9	0.0		
			min	0.0	0.0	-76.7	13.4	-4.9	0.0		
	114	2.090	Max uX	0.0	0.0	-76.7	13.4	-4.9	0.0	CO 94	
	2529	0.000	Min ux	0.0	0.0	48.8	13.2	4.9	0.0	CO 97	
	2529	0.000	MaxuY	0.0	0.0	-48.8	13.2	-4.9	0.0	CO 97	
	2529	0.000	Min uY	0.0	0.0	-48.8	13.2	-4.9	0.0	CO 90	
_	2529	0.000	Maxuz	0.0	0.0	-48.8	13.2	-4.9	0.0	CO 89	
_	114	2.090	Min uz	0.0	0.0	-/6./	13.4	-4.9	0.0	CO 85	
-	2520	2.090	Μίρ φχ	0.0	0.0	-/0./	13.4	-4.9	0.0	0.90	
-	2529	0.000	Μαν φΥ	0.0	0.0	-40.0	13.2	-4.9	0.0	0.00	
-	2529	0.000	Μίη ωΥ	0.0	0.0	-48.8	13.2	-4.9	0.0	CO 97	
-	2529	0.000	Max oz	0.0	0.0	-48.8	13.2	-4.9	0.0	CO 85	
-	2529	0.000	Min oZ	0.0	0 0 0	-48.8	13.2	-4.9	0.0	CO 89	-
	2661	0.000	max	0.0	0.0	-6.3	1.3	2.0	0.0		
			min	0.0	0.0	-6.3	1.3	2.0	0.0		
	117	2.090	max	0.0	0.0	-9.3	1.5	2.0	0.0		
			min	-0.1	0.0	-9,4	1.5	2.0	0.0		
	2661	0.000	Max uX	0.0	0.0	-6.3	1.3	2.0	0.0	CO 98	
	117	2.090	Min ux	-0.1	0.0	-9.3	1.5	2.0	0.0	CO 89	
_	2661	0.000	Max uY	0.0	0.0	-6.3	1.3	2.0	0.0	CO 89	
_	2661	0.000	MinuY	0.0	0.0	-6.3	1.3	2.0	0.0	CO 85	
_	2661	0.000	Maxuz	0.0	0.0	-6.3	1.3	2.0	0.0	CO 90	
_	117	2.090	Min uz	0.0	0.0	-9.4	1.5	2.0	0.0	CO 84	
-	2661	2.090	Min @Y	0.0	0.0	-9.4	1.5	2.0	0.0	00.04	
-	2661	0.000	Max (cV	0.0	0.0	-0.3	1.3	2.0	0.0	CO 90	
-	2661	0.000	Min oY	0.0	) 0.0	-6.3	1.3	2.0	0.0	CO 84	
	2661	0.000	Max oz	0.0	0.0	-6.3	1.3	2.0	0.0	CO 90	
	2661	0.000	Min øZ	0.0	0.0	-6.3	1.3	2.0	0.0	CO 84	
ore											



Figure 25. Local deformations

# 8.3 Support reactions

It is also possible to evaluate support reactions graphically or in tables. Support reactions are listed in lines and are related to the start node of the line. The surface FE mesh is not relevant to the line support forces. Presented in results, supporting reactions are generated for the individual load case and combination.

### 8.4 Internal forces

Internal forces are axial forces, shear forces and moments acting within a body. Presented both graphically and numerically internal forces are sorted by surfaces or members. The difference between surface internal forces and member internal forces is that a member moment M rotates about the local axis of a member, whereas surface moment acts in the direction of local surface axis.

### 8.5 Structural components design

In addition to program's ability to determine deformations, support reactions and internal forces, RFEM provides several add-on modules, which can perform structural design of elements consisting of different materials and according to specific standards.

The RF-CONCRETE add-on module is used to determine cracked sections in reinforced concrete structures and to design structural components. The module consists of two parts: RF-CONCRETE Members and RF-CONCRETE Surfaces.

### 8.5.1 RF-CONCRETE

Members designed through RF-Concrete are imported directly from RFEM. Loads, load combinations and result combinations are then assigned to determine internal forces of selected elements. The library covers the property of materials of the relevant standard and can be altered if necessary (Figure 26).

	Δ	В	C	1	h	Concrete Stress-Strain Curve
IcineteM	Material De	escription			<i>,</i>	Concrete Stress-Strain Surve
No.	Concrete Strength Class	Reinforcing Steel	Notes	Com	ment	
2	Concrete C25/30	B 500 S (A)	10003	Com	inom	
2	0010101010 020100	D 300 3 (A)				σ <sub>c</sub> (<0) ▲
						-[4
						En En elet
						6c2 6c2u 6c (<
					2	8
						Reinforcement Stress-Strain Curve
Material	Properties					
	rete Strength Class: Concrete	e C25/30				
Char	acteristic Cylinder Compressive S	trength fck	25	N/mm <sup>2</sup>		
Mea	n Cylinder Compressive Strength	fcm	33	N/mm <sup>2</sup>		first fits, cal
Mea	n Axial Tensile Strength	fctm	2.6	N/mm <sup>2</sup>		fu/7: fkal/7s
- 5% F	ractile of Axial Tensile Strength	fctk;0.05	1.8	N/mm <sup>2</sup>		
95%	Fractile of Axial Tensile Strength	fctk;0.95	3.3	N/mm <sup>2</sup>		arctan E.
Mea	n Secant Modulus of Elasticity	Ecm	31000	N/mm <sup>2</sup>		
BChar	acteristic Strains for Nonlinear Ar	alysis	1 1000			Esu Es
Ult	imate Strain for Pure Compressio	n ɛc1	-2.1	%0		
Ult	imate Strain at Failure	8c1u	-3.5	%0		
BCha	acteristic Strains for Parabolic-Re	ectangular Diagram	1	-		Material No. 2 used in
Ult	imate Strain for Pure Compressio	n εc2	-2	%0		
Ult	imate Strain at Failure	ec2u	-3.5	%0		Cross-sections:
Ult	imate Strain at Failure	εc3	-1.75	%		1,2
Ult	imate Strain at Failure	εc3u	-3.5	%0		
Pa	rabola Exponent	n	2			Members:
Spee	cific weight	γ	25	kN/m <sup>3</sup>		
Reinf	orcing Steel: B 500 S (A)		1			121-125,127,129,130,132-135,141-146
Mod	ulus of Elasticity	Es	200000	N/mm <sup>2</sup>		
Char	acteristic Yield Strength	fyk	500	N/mm <sup>2</sup>		Sets of members:
Char	acteristic Tensile Strength	ftk	525	N/mm∠		
Ultin	nate Strain	εuk	25	%0		
						T Landby T Mainte
						a conyrit. a wogin.

Figure 26. Material Properties

The RF-Concrete Member add-on is used to design reinforcement for beams, columns and other elements, where RF-Concrete Surfaces is used to design reinforcement for surface. Prior to design calculation, construction properties are set to determine the required longitudinal and shear reinforcement. During this project possible diameters of longitudinal reinforcement are set from 16 to 28 mm and from 8 to 10 mm for stirrups. The results of design are then presented both in tables and graphically, visualizing the most suitable solution (see Figure 27). Structural designs generated by RF-Concrete can be seen in Appendix 5.

Item         Reenforcement         No. of         ds         As         Length         -x-Locator (m)         Weight           No         Position         Bars         [mm]         [cm²]         [m]         from         to         Anchorage         [kg]         Notes           Member No. 067         Rectange 000300	
No.         Position         Bars         [mn]         [mn]         from         to         Anchorage         [kg]         Notes           Martie N. volgers         76 / 75. Redcargle 200300         2         20.0         6.28         0.650         -0.200         5.855         QJ         2.90.4         159)           ent         1         -2 forp)         2         20.0         6.28         0.050         QJ20         5.855         QJ         2.90.4         159)	
Image: No. 987 - Rectangle 300:300           1         -z (top)         2         20.0         6.28         6.050         -0.200         5.850         ☑         29.84         156)           2         +z (totothem)         2         20.0         6.28         6.050         0.200         5.850         ☑         29.84         156)	
nt 1 -z (top) 2 200 6.28 6.050 -0.200 5.850 2 29.84 (56) 2 +z (bottom) 2 200 6.28 6.050 -0.200 5.850 2 29.84 (58)	
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2 +z (bottom) 2 20.0 6.28 3.700 -0.200 3.500 🔽 18.25 158)	
ation Member No. 971 - Rectangle 300/300	
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(Course)	
() 2 ± 23.3, i = 6.003 m	

Figure 27. Provided reinforcement

#### 8.6 Updating existing BIM

To complete building information modelling, structural analysis is exported from Dlubal RFEM to Autodesk Revit. The program's bidirectional data exchange makes it possible to create new elements and adjust modifications in both directions. Adjustable reference lines allow load bearing structural parts to merge automatically in a single model. If changes are made to cross-sections or materials, the data is then updated in both programs. New structural components are added, and modified designs are removed as seen in Figure 28.

The results of structural analysis such as internal forces, deformations and defined reinforcement groups can also be imported to Revit. A corresponding view as well as Result Explorer toolkit is generated automatically and is available after import is complete. Manual adjustments of the analytical model are always possible in case of occurred inaccuracies. Exported elements of the analytical model can be found in Appendix 6.



Figure 28. Exported reinforcement

### 9 CONCLUSION

The availability of information online and provision of free student versions of the programs by Autodesk and Dlubal software, makes it possible for those who are interested, to learn operating the programs efficiently. This makes the studying process relatively fast and easy.

Revit and RFEM mostly circulate amongst large construction companies, but most, if not all the features and libraries are accessible in student and trial versions of programs, for separate users to experience the software prior to its purchase. Programs used during this project not only provided results for structural analysis and combination of them in a BIM, but also gave insights into learning to use Revit and RFEM and laid the basis for a potential future renovation.

The presence of link between two programs allowed the project to expand its possibilities. The program's tools effectively assisted in conflict determination and its further elimination, making the next step of the process possible when certain condition is fulfilled. Although there is always room for improvement, the approach taken during designing stage of the project is represented through a flowchart seen in Figure 29, compiled for better understanding of the workflow.



Figure 29. Flowchart of the project

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Appendix 1/1 Visible crack on the old concrete slab



Appendix 1/2 Cracks formed around old ventilation shaft





Appendix 1/3 Reinforcement of the old the damaged concrete slab

Appendix 1/4 Reinforcement of deteriorating concrete slab



Appendix 2/1 Autodesk Revit generated model and drawings Demolition drawing of attic structures



Appendix 2/2 Layout of masonry walls and old INP 200-profiles



Appendix 2/3 Masonry wall and intermediate floor section view



### 1 OLEMASSA OLEVA KIVIRAKENTEINEN SEINÄ

1:10



1 OLEMASSA OLEVAT I-PALKIT

2 BETONIALALAATTA 150 MM

34



Appendix 2/4 Layout of the new structural frame 2-5 floor

Appendix 2/5 Structural model of the building

Appendix 3/1 Dlubal RFEM generated analytical model model Dead loads





Appendix 3/2 Live load



Appendix 3/3 Snow load



Appendix 3/4 Wind Load South



Appendix 3/5



Appendix 4/1 Structural analysis results Global deformations





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Appendix 4/3



Appendix 4/4









Appendix 5/1 RF-Concrete structural designs RF-Concrete generated reinforced concrete members







Appendix 5/3 Reinforced beam



Appendix 6/1 Revit reinforced section view



Appendix 6/2 Revit reinforced Column



1.

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Appendix 6/3 Revit reinforced beam



